

A combined approach to cliff characterization: Cliff Stability Index

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Abstract

In this work, a combined multidisciplinary method to characterize coastal cliff environments is presented. It combined two complementary approaches – engineering and geomorphological. The first one is represented by the wave power values along the cliff face. For that purpose, the deep-water wave climate is statistically characterized, and high-energy sea states are numerically propagated to the cliff with a state-of-the-art model. The variations in wave power at the cliff face are controlled by the varying cliff orientation and by the irregular morphology, which influences wave propagation through refraction and shoaling processes. Based on wave power, four engineering exposure levels, from low to extreme, are defined and mapped onto the cliff. The geomorphological approach is based on an index developed *ad hoc* for this work, the Cliff Stability (CS) index, which takes into account the cliff geometry, lithology, structure and degradation state, as well as the hydrological conditions. Based on the CS index, four geomorphological exposure levels are defined and mapped, from low to extreme. The combined approach is shown through the application to a study

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site in NW Spain. The two perspectives, engineering and geomorphological, are found to yield similar results in some sections of the study area, but not all. It may be inferred that, despite the importance of wave action in shaping the cliff, the additional elements included in the CS index also play a significant role. In practical terms, the significance of these results is that the two approaches, engineering and geomorphological, should be combined to properly characterize coastal cliffs. This combined approach represents a multidisciplinary tool to define and characterize the exposure levels and, thus, prevent damages in cliff environments across the world.

Keywords: Coastal geology; coastal geomorphology; cliff stability; wave power; exposure.

1. Introduction

Coastal cliffs are the most frequent common coastal landforms, representing about 75% of the coastlines around the world (Emery and Kuhn, 1982; Bird, 2011). Some of these cliff coasts represent geological heritage with a great tourist value (Fig. 1), such as the Twelve Apostles (Australia), the Minamijima Island (Japan), the Pigeon Rocks (Lebanon), the Cathedral (Peru), the Darwin Arch (Ecuador), the Portada (Chile), the Arco de Cabo San Lucas (Mexico), the Holei Sea Arch and Sunsent Cliffs (United States), the Perce Rock (Canada), the Hvitserkur (Iceland), the Cliffs of Moher (Republic of Ireland), the Arch Rock in Arnarstap (Ireland), the Stacks of Duncansby, Yesnaby and Thirle Door (Scotland), the Green Bridge (Wales), the Durdle Door, Flamborough Head and Old Harry Rocks (England), the Azure Window (Malta), the Drangarnir (Faroe Islands), the Porte d'Aval (France), the Quebrada coast, Cuevas del Mar beach and Cathedral beach (Spain), or the Ponte da Piedade (Portugal), among others.



Figure 1: From left to right, top to bottom: Azure Window, Arco de Cabo San Lucas, Ponte da Piedade, Minamijima Island (first row); Durdle Door, Perce Rock, Arch Rock, Cuevas del Mar (second row); Portada, Porte d'Aval, Moher Cliffs, Quebrada coast (third row); Twelve Apostles, Pigeon Rocks, Green Bridge, Yesnaby (fourth row); Flamborough Head, Thirle Door, Holei Sea Arch, Hvitserkur (fifth row).

15 Numerous works have studied erosion problems on cliff coasts over the last
16 decades (Sunamura, 1977; Robinson, 1977; Jones and Williams, 1991; Komar
17 and Shih, 1993; Shih and Komar, 1994; Duperret et al., 2002; Moore and Griggs,
18 2002; Sallenger Jr et al., 2002; Rosser et al., 2005; Hansom et al., 2008; Dawson
19 et al., 2009; Hapke and Plant, 2010; Lim et al., 2011; De Rose and Basher,
20 2011; Barlow et al., 2012; Dickson et al., 2013; Carpenter et al., 2014; Jones
21 et al., 2015; Johnstone et al., 2016; del Río et al., 2016; Pappalardo et al., 2017;
22 Earlie et al., 2018; Prémaillon et al., 2018; Westoby et al., 2018; Terefenko et al.,
23 2018, 2019; Zelaya Wziatek et al., 2019; Alessio and Keller, 2020; Muñoz-López
24 et al., 2020; Di Crescenzo et al., 2021). For example, Sunamura (1977) proposed,
25 based on field and laboratory data, a relationship between the wave force and
26 the resulting cliff erosion. Later, Jones and Williams (1991) analysed the factors
27 that induce cliff erosion on the Welsh coast through modelling of wave refraction,
28 measurements of annual recession and regression approach. Cliff erosion was also
29 studied during the 90s by means of local controls (Komar and Shih, 1993) and
30 field measurements along the Oregon coast (Shih and Komar, 1994).

31 In the first decade of this century, Duperret et al. (2002) analysed the col-
32 lapse of a cliff through stratigraphical dating along with observations and field
33 measurements in France; Rosser et al. (2005) used terrestrial laser scanning to
34 measure and characterize cliff processes in the UK; Dawson et al. (2009) in-
35 vestigated the effects of sediments detached from cliff erosion in the protection
36 of low-lying coastal areas against flooding; and Ogawa et al. (2011) described
37 wave transformation processes across a shore platform based on a field exper-
38 iment conducted in New Zealand. Other works on cliff erosion over the last
39 ten years have been based on the implementation of models to simulate the
40 erosion of cliffs (Hapke and Plant, 2010; Barlow et al., 2012; Terefenko et al.,
41 2019; Muñoz-López et al., 2020); the study of aerial photographs (De Rose and
42 Basher, 2011; del Río et al., 2016), multiview stereos (Westoby et al., 2018),
43 videos with high resolution (Thompson et al., 2019), and terrestrial and aerial
44 vehicle photometry (Letortu et al., 2018); and the analysis of laser scanning
45 measurements (Lim et al., 2011; De Rose and Basher, 2011; Johnstone et al.,

46 2016; Earlie et al., 2018; Westoby et al., 2018; Terefenko et al., 2018, 2019;
47 Zelaya Wziatek et al., 2019; Alessio and Keller, 2020).

48 This work proposes a novel method to characterize coastal cliff environments,
49 which combines geomorphological and engineering approaches. The combined
50 approach is used to characterize a study site in north-western Iberian Peninsula.
51 The engineering approach, which is represented by the wave power impinging on
52 the cliff, covers the statistical analysis of deep-water wave climate, the numerical
53 wave propagation toward the cliff of relevant sea states, and the assessment of
54 wave power values on the cliff face. The geomorphological approach is based
55 on a novel index, defined *ad hoc*: the Cliff Stability (CS) index, including the
56 characterization of the cliff geometry, lithology, structure, degradation state and
57 hydrological conditions.

58 2. Study site

59 The Catedrales cliff and beach are located in the Atlantic Ocean littoral
60 of northwest Spain, in the Iberian Peninsula (Figs. 2 and 3). The stretch of
61 coastline considered in this study has a length of 1700 m. This area, of great
62 environmental and tourism value, was registered as Natural Monument in 2005.



Figure 2: (a) Study site location (north-western Iberian Peninsula). (b) Plan view of the Catedrales cliff and beach.

63 The Catedrales beach is located to the West of the basal thrust of the Manto
64 de Mondoñedo and more specifically between the Eo anticline and the Villao-
65 drid syncline. The rocks of the Catedrales beach belong to the Los Cabos



Figure 3: Catedrales Cliffs.

66 formation, and in particular to the Brens layer (Bastida and Pulgar, 1978).
67 The stratigraphic profiles present on the cliffs are formed by levels of sandstone
68 with insertions of siltstone layers, under which a harder and more resistant se-
69 ries formed by quartzite and gray slate is located, which has somewhat greater
70 thickness and may appear in alternation. These profiles can be affected by
71 other geological structures associated with existing tectonics, such as main or
72 secondary faults, minor folds and shear zones related with sub-horizontal thrust
73 faults.

74 All along the cliff there are several strongly-inclined-plane faults of unequal
75 relevance, in which two main groups of directions stand out: NNE-SSW and
76 WNW-ESE. The analysis of the existing joint families allows determining the
77 strong tectonic control acting all along the cliff, which has determined the geo-
78 morphological units related to the cliff evolution itself. The structural arrange-
79 ment has conditioned the coastline morphology as well as the appearance of all
80 existing geomorphological units, such as galleries, caves, blowholes, cliffs, stacks,
81 etc.

82 The wave climate in the study site is highly energetic due to its location,
83 exposed to the long Atlantic fetch (Carballo et al., 2015a,b; Veigas et al., 2015).
84 The mean wave direction is north-west (Fig. 4). Under storm conditions, signif-
85 icant wave heights in deep water exceed ten meters. These storm waves erode
86 the cliff, causing material damages and, tragically, loss of human life. A tourist
87 was killed in March 2018 by a stone falling from the cliff.

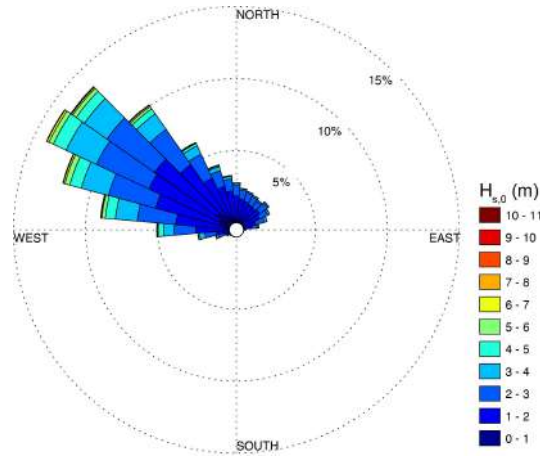


Figure 4: Deep-water wave rose at the study site. Data source: ERA5 model

88 **3. Methods**

89 *3.1. Wave data and treatment*

90 *3.1.1. Deep-water wave climate*

91 The Peak Over Threshold (POT) method (Goda, 2010) was used to statis-
 92 tically characterise the extreme regime of deep-water significant wave heights.
 93 For this purpose, the data series of the ERA5 model of the ECMWF (Euro-
 94 pean Centre for Medium-Range Weather Forecast) were employed. To apply
 95 the POT method, a threshold value equal to the significant wave height that
 96 is exceeded 1% of the time (i.e., $H_T = H_{99\%}$) was considered. After the Peak
 97 Over Threshold method was applied, several theoretical cumulative distribution
 98 functions were fitted to selected extreme significant wave height values.

99 *3.1.2. Numerical wave propagation*

100 The wave propagation model *SWAN* (Holthuijsen et al., 1993; Booij et al.,
 101 1999) was used to propagate the sea states detailed in the previous section under
 102 high-tide conditions, from deep water (ERA5 node location, Fig. 5) towards the
 103 cliff toe. This numerical wave propagation model has been used for a wide range
 104 of coastal engineering applications in the past few years (Bergillos et al., 2016a;
 105 López-Ruiz et al., 2016a; Bergillos et al., 2016b, 2017b,a; López-Ruiz et al.,

106 2016b; Bergillos et al., 2018b,a; Magaña et al., 2018; López-Ruiz et al., 2018b,a;
 107 Rodríguez-Delgado et al., 2018a,b, 2019b; Bergillos et al., 2019c,a,b; Rodríguez-
 108 Delgado et al., 2019c,a, 2020; Rodríguez-Delgado and Bergillos, 2021).

109 To set up and apply the wave propagation model, bathymetric and topo-
 110 graphic data collected over a field campaign were used. The bathymetric mea-
 111 surements were complemented by bathymetric data of the European Marine
 112 Observation and Data network, EMODnet (Thierry et al., 2019). Two numeri-
 113 cal grids were also defined and employed (Fig. 5): a coarse grid along the whole
 114 numerical domain and a nested grid located only at the nearshore zone, with a
 115 lower plan view area but a greater resolution.

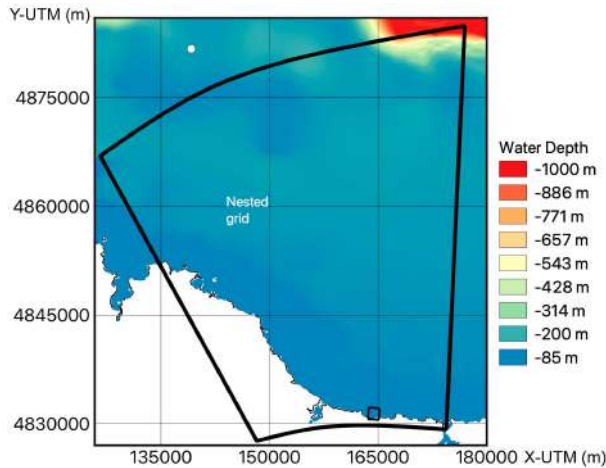


Figure 5: Water depths (in m), computational grids used for the *SWAN* numerical model, and ERA5 point location.

116 3.1.3. Wave power acting on the cliff face

117 The results obtained with *SWAN* were used to compute wave power through
 118 the equation (Astariz and Iglesias, 2016a,b):

$$P = \frac{1}{16} \rho g H_s^2 C_g, \quad (1)$$

119 where g is acceleration of gravity, H_s is significant wave height, ρ is density of

120 water, and C_g is celerity of the wave group, computed as (Besio et al., 2016;
121 Contestabile et al., 2017):

$$C_g = \frac{c}{2} \left(1 + \frac{2kh}{\sinh(2kh)} \right) \sqrt{\frac{g}{h} \tanh(kh)}, \quad (2)$$

122 where h is water depth, c is wave celerity, and k is wave number.

123 3.1.4. Definition and mapping of exposure levels

124 The following exposure levels were defined according to the engineering ap-
125 proach:

- 126 • Low exposure (LE): Wave power < 4.6 kW/m,
- 127 • Middle exposure (ME): $4.6 \leq$ Wave power < 9.2 kW/m,
- 128 • High exposure (HE): $9.2 \leq$ Wave power < 13.8 kW/m,
- 129 • Extreme exposure (EE): Wave power \geq 13.8 kW/m.

130 These exposure levels were mapped along the study area.

131 3.2. Geomorphological approach and Cliff Stability (CS) index

132 The geomorphological approach was based on the novel Cliff Stability (CS)
133 index, defined *ad hoc* as the weighted sum of factors relating to the geometry,
134 lithology, structure, degradation state and hydrological conditions of the cliff.
135 Whereas the first four factors (geometry, lithology, structure and degradation
136 state) are related to the properties of the cliff, the hydrological conditions (pre-
137 cipitation, wave action) depend not only on the properties of the cliff (e.g.,
138 infiltration, orientation) but also on the cliff location.

139 The Cliff Stability (CS) index is defined as

$$CS = (G + L + E_1 + E_2 + D) \cdot P \cdot W, \quad (3)$$

140 where G represents the influence of the geometry, L the lithology, E_1 the macro-
141 structure, E_2 the micro-structure, D the degradation state, P the precipitation
142 and W the wave action.

143 The maximum value of the CS index was set to 100, by definition. The
144 ranges of the factors in Equation (3) were chosen to represent the influence of
145 each factor on the cliff stability. The maximum values established for P and
146 W were 1.11 and 1.5, respectively. As regards the other factors, the following
147 ranges were considered: G varies between 0 and 4, L between 0 and 14, E_1
148 between 0 and 8, E_2 between 0 and 16, and D between 0 and 18. Thus, the
149 maximum value of the sum of factors in brackets in Equation (3) is 60. The
150 evaluation of these factors based on the field campaign is explained below. The
151 field campaign was meticulously carried out by two experienced geologists over
152 three months.

153 The study site was divided into 24 zones with similar characteristics and
154 conditions (Fig. 6). In each zone, the CS index was quantified at the upper and
155 lower parts of the cliff face through *in situ* observations. The CS assigned to
156 each zone is the maximum value of the two obtained for the upper and lower
157 parts.



Figure 6: Zonification for the geomorphological approach.

158 3.2.1. Geometry

159 The geometry factor (G) is obtained based on the height of the cliff and the
160 type of cliff profile (Table 1). Thus, in the stretches with cliff heights below 6

161 m, G varies between 1 (for vertical cliff profiles or cliff profiles leaning toward
 162 the land) and 2 (for cliff profiles leaning toward the sea). For stretches of cliff
 163 higher than 6 m and below 12 m, G varies between 1.5 m and 3 m. Finally, for
 164 cliffs with heights greater than 12 m, G is between 2 and 4.

Table 1: Values of the geometry factor (G) depending on the height of the cliff (H) and the cliff profile.

Cliff profile	$H \leq 6$ m	$6 < H \leq 12$ m	$H > 12$ m
Vertical or leaning towards the land	1	1.5	2
Leaning towards the sea	1.5	2.5	3
With undermining at the lower part	2	3	4

165 3.2.2. Lithology

166 The rocks of the Aguasantas cliff belong to the Los Cabos Formation, and
 167 in particular to the Brens layer, where there is an alternation of quartzites,
 168 sandstones, siltstones and slates. The lithology factor (L) depends on the rock
 169 layer thickness and also on the typology of the rock: hard and abrasion resistant
 170 (e.g., quartzites and quartz schists), soft and erodible (e.g., lightly cemented
 171 sandstones and phyllites) or mixed (Table 2).

Table 2: Values of the lithology factor (L) as a function of the rock layer thickness (S) and the rock type.

Rock type	$S \leq 0.3$ m	$S > 0.3$ m
Hard and abrasion resistant	6	3
Soft and erodible	14	9
Mixed	10	6

172 3.2.3. Structure

173 Two types of structure factors are considered: macro-structure (E_1 , Table 3),
 174 like large cracks or open vertical joints, and micro-structure (E_2 , Table 4), which
 175 takes into account the number of discontinuities and the type of discontinuities
 176 that weaken the rock mass (apart from those considered by E_1).

Table 3: Values of the macro-structure factor (E_1) as a function of the main stratification.

Main stratification	E_1
Subvertical and parallel to the slope	8
Subvertical and perpendicular to the slope	2
Subhorizontal	6
Oblique with variable inclination	4

Table 4: Values of the micro-structure factor (E_2) as a function of the type of joints (smooth, rough or filled) and the frequency of the joints.

Frequency of the joints	Smooth joints	Rough joints	Filled joints
Low frequency	6-10	4-6	6-8
Middle frequency	8-12	6-8	8-10
High frequency	10-16	8-12	9-14

177 *3.2.4. Degradation state*

178 The degradation factor (D) was obtained through the characterization of
 179 several significant phenomena observed: the rockfalls, cornices at the top of the
 180 cliffs, opened cracks, relevant landslides, the volume of caves and the temporal
 181 evolution of the fauna and flora attached to the cliff (which is indicative of the
 182 temporal evolution of the degradation), as it is indicated in Table 5.

Table 5: Values of the degradation state factor (D) as a function of the degree of degradation, temporal evolution, rockfalls volume, frequency of cornices and volume of caves.

Degree of degradation	Temporal evolution	Volume of rockfalls			Frequency of cornices			Volume of caves		
		High	Middle	Low	High	Middle	Low	High	Middle	Low
Null or light	Insignificant	6	4	2	6	4	2	8	6	4
	Short term	10	8	6	10	8	6	12	10	8
	Long term	8	6	4	8	6	4	10	8	6
Middle	Insignificant	10	8	6	10	8	6	12	10	8
	Short term	14	12	10	14	12	10	16	14	12
	Long term	12	10	8	12	10	8	14	12	10
High	Insignificant	12	10	8	12	10	8	14	12	10
	Short term	16	14	12	16	14	12	18	16	14
	Long term	14	12	10	14	12	10	16	14	12

183 *3.2.5. Hydrological conditions*

184 Two hydrological conditions are considered to assess the CS index: precip-
 185 itation and wave power. The assessment of the effects of the water on the cliff
 186 was performed after an intense rain. The value of the precipitation factor (P)
 187 depends on the runoff from the crowning of the cliff, and the presence or absence
 188 of corrective actions, such as sealing layers (Table 6).

Table 6: Values of the precipitation factor (P) as a function of the runoff from the crowning of the cliff and the corrective actions.

Runoff	With corrective actions	Without corrective actions
Null	1	1
Middle	1	1.05
Important	1.05	1.11

189 The wave power factor (W) was quantified based on the wave exposure
 190 levels obtained with the engineering approach and the type of sediment (sand
 191 or gravel), as it is indicated in Table 7.

Table 7: Values of the wave power factor (W) as a function of the sediment type at the cliff toe and the wave exposure (low, middle, high or extreme).

Sediment type	Low exp.	Middle exp.	High exp.	Extreme exp.
Sand	1	1.1	1.2	1.3
Gravel	1	1.2	1.35	1.5

192 *3.2.6. Geomorphological mapping of exposure levels*

193 The exposure levels in the geomorphological approach were established on
 194 the basis of the CS index defined by Eq. (3). Four geomorphological exposure
 195 levels were established (Table 8) and these levels were mapped along the study
 196 site.

Table 8: Geomorphological exposure levels as a function of the CS index values.

Exposure level	CS range
Low exposure	$CS < 25$
Middle exposure	$25 < CS < 50$
High exposure	$50 < CS < 75$
Extreme exposure	$CS > 75$

197 **4. Results**

198 Among the theoretical cumulative distribution functions (CDF) tested, the
 199 best fit to the extreme significant wave height values was obtained with by the
 200 Generalized Extreme Value function. Based on this Generalized Extreme Value
 201 CDF, the values of wave height for return periods of 2 years, 10 years, 50 years
 202 and 100 years were calculated to be 6.6 m, 8.3 m, 11 m and 12.7 m, respectively
 203 (Fig. 7).

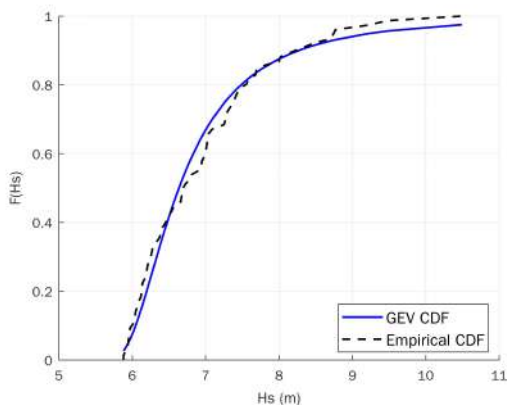


Figure 7: Generalised Extreme Value function and empirical CDF of the significant wave height values (H_s) above the threshold.

204 These significant wave heights in deep water, the most typical peak period
 205 under storm conditions and the most frequent incoming mean direction un-
 206 der high-energy conditions ($T_p=15$ s and $\theta=300^\circ$) were numerically propagated
 207 toward the cliff toe for high-tide conditions by means of the *SWAN* model, pre-
 208 sented in Section 3.1.2. Fig. 8 shows the significant wave heights within the

209 nested grid for the four return periods considered: 2 years, 10 years, 50 years
 210 and 100 years. Deep-water significant wave heights range from 3.3 m to 5.5
 211 m, for return periods of 2 and 100 years, respectively. This dependence of the
 212 significant wave height upon the return period occurs in deep and transitional
 213 water but not in the shallow water in front of the cliff due to wave breaking in-
 214 duced by depth (Fig. 6). In the shallow water nearshore there is some variation
 215 in the wave height values at the cliff face. This is induced by wave refraction
 216 and shoaling due to the irregular bathymetry. In particular, close to the west
 217 boundary of the nested grid, the lower water depths lead to wave breaking at
 218 longer distances from the cliff toe, resulting in an area of lower significant wave
 219 heights.

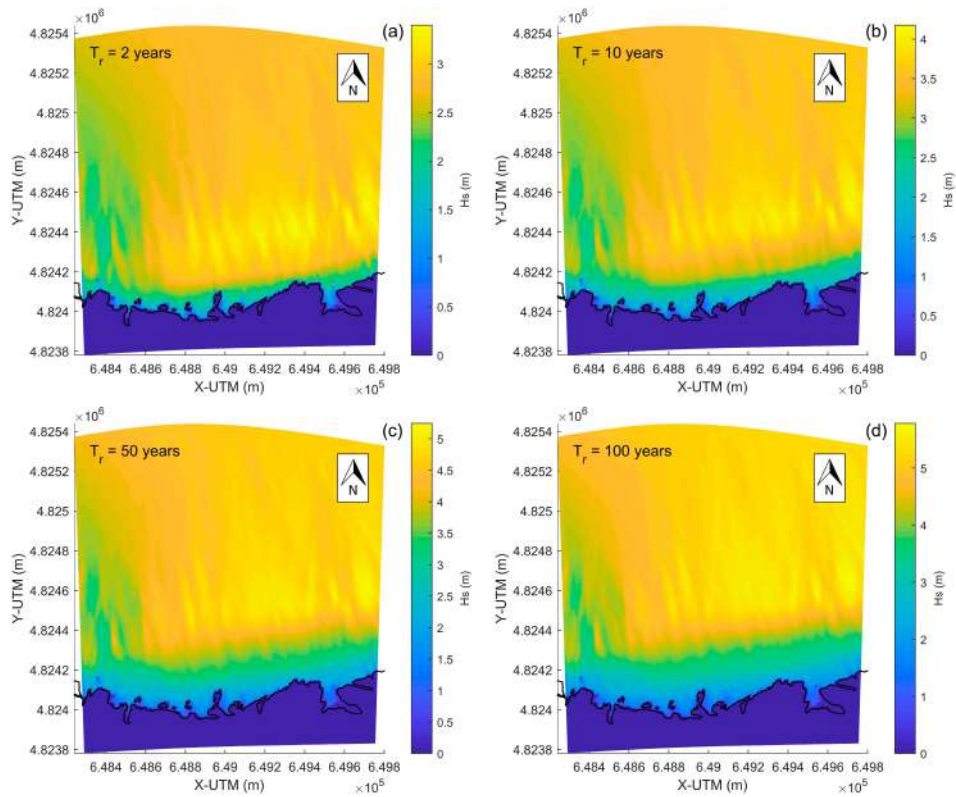


Figure 8: Significant wave height distributions in the nested grid for return periods of 2, 10, 50 and 100 years.

220 The values of wave power on the cliff face were quantified for the deep-water
 221 significant wave height value corresponding to a return period of 2 years (i.e.,
 222 $H_{s,0} = 6.6$ m) given that, as reported in the previous paragraph, the wave height
 223 values at the cliff face do not vary for the different return periods (Fig. 8). Fig.
 224 9a shows the wave power distribution in the nested grid. The wave breaking
 225 induced by water depth reduces the wave power at the cliff face. In the western
 226 part of the nested grid the lower water depths reduce the significant wave heights
 227 in that area (Fig. 8) and, therefore, the wave power (Fig. 9a).

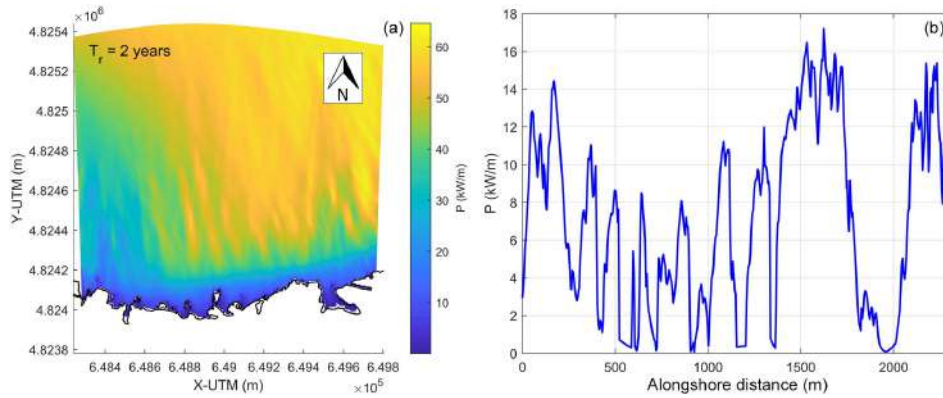


Figure 9: Wave power distribution in the nested grid (a) and variability of wave power values along the cliff face (b) for a 2-year return period.

228 The right panel of Fig. 9 presents the distribution of wave power values at the
 229 cliff toe. This distribution is highly variable due to the variation in significant
 230 wave heights at the cliff face, which is induced, in turn, by the different cliff
 231 orientations and complex bathymetry of the study site. The irregular wave
 232 power distribution on the cliff leads to different levels of cliff exposure from the
 233 engineering point of view.

234 The mapping of the exposure levels obtained with the engineering approach
 235 along the study site is shown in Fig. 10. It may be observed that exposure levels
 236 are ultimately controlled by the cliff orientation and the nearshore bathymetry,
 237 which influences the variations in significant wave heights (and other wave prop-
 238 erties) in the propagation from deep water toward the cliff and the resulting
 239 wave power distribution at the cliff face. These wave power variations lead to
 240 the different exposure levels from the engineering point of view (Fig. 10). For
 241 instance, the inlet close to the east boundary of the study site is a LE zone since
 242 the wave power values in this area (Fig. 9b, $s \approx 2000$ m) are low.



Figure 10: Exposure levels according to the engineering approach.

243 The Pena dos Corvos Island (west zone of the study site; Fig. 10) has HE and
 244 EE levels on its north face and LE level on its south face due to the protection
 245 provided by the island and the higher water depths on the north face. The
 246 Xangal Island (central zone of the study site; Fig. 7) leads to a similar exposure
 247 variability, with HE and EE levels in the windward face and a LE level in the
 248 leeward face. In the study area there are also some isolated rocks near the
 249 cliff face, which act as submerged (emerged) marine structures for high (low)
 250 astronomical tides, reducing the wave power values impinging on the cliff. This
 251 mitigation of wave power is, obviously, less effective than that provided by the
 252 aforementioned islets.

253 On the other hand, based on the exposure levels obtained by means of the
 254 geomorphological approach (Fig. 11), the exposure of the cliffs located in the
 255 western zone of the study site is high. This high exposure is extended until
 256 the central stretch of beach, with only a small zone of middle exposure in the
 257 eastern part of the inlet (zone E-2, Fig. 6). In the central stretch of beach the
 258 level of exposure is generally low or middle, whereas in the Xangal Islet the
 259 exposure is high. From this section of beach toward the eastern boundary, the
 260 exposure is middle to high, except a small section of low exposure at the eastern
 261 inlet (zone E-23, Fig. 6).

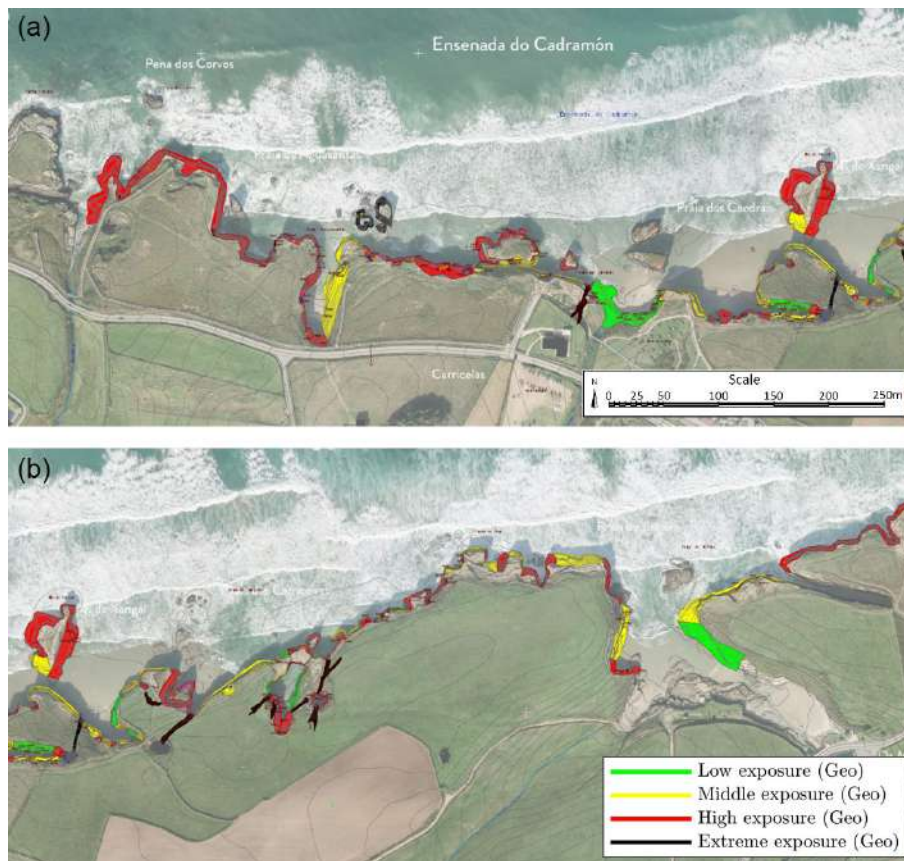


Figure 11: Exposure levels according to the geomorphological approach: (a) From zone E-1 to zone E-13, (b) from zone E-14 to zone E-24.

262 5. Discussion

263 A previous approach to the problem addressed in this work was made by
264 Gornitz et al. (1991), who proposed the first Coastal Vulnerability Index (CVI),
265 composed of seven variables as indicators of physical vulnerability due to the
266 sea level rise. These variables consisted of relief (elevation), lithology, geo-
267 morphology, erosion/accretion, tidal range, wave height, and relative sea-level
268 changes. This methodology was improved later by Gornitz et al. (1994), who
269 added new variables as permanent inundation (elevation and local subsidence)
270 and episodic inundation (tropical storm probability, hurricane probability, hur-
271 ricane frequency-intensity, etc.). However, this approach was mainly oriented to
272 the assessment of coastal vulnerability due to future sea level rise from green-
273 house climate warming. Conversely, the CS index presented in this paper focuses
274 mainly on the current geological and marine conditions because these cliffs are
275 visited annually by more than 500,000 people and it is necessary to estimate the
276 current vulnerability of the cliffs. Furthermore, the CS index takes into account
277 some variables which are not included in the CVI and which play a relevant role
278 in the particular case of the Catedrales cliffs: the macro and microstructure of
279 the rock formations, the rainfall and runoff from the crowning of the cliff, the
280 sediment type at the cliff toe, the wave exposure level and the degradation state
281 of the cliffs.

282 When the results of both approaches (engineering and geomorphological)
283 are compared, a clear parallelism is apparent. There are, of course, some differ-
284 ences, which might have been expected given that both approaches are based
285 on different indicators – wave power and Cliff Stability (CS) index. At any
286 rate, these differences are not greater than one exposure level, excepting four
287 small zones which have been marked and numbered in Fig. 12. In the cases
288 marked as (1), (2) and (4) the geomorphological exposure is higher because of
289 the abundant rockfalls resulting from erosive phenomena of non-marine origin.
290 By contrast, in the case marked as (3) the engineering exposure is greater be-
291 cause the lithology corresponds to a homogeneous profile formed by the hardest

292 and more resistant rocks in the zone, such as quartzites and quartz schists.

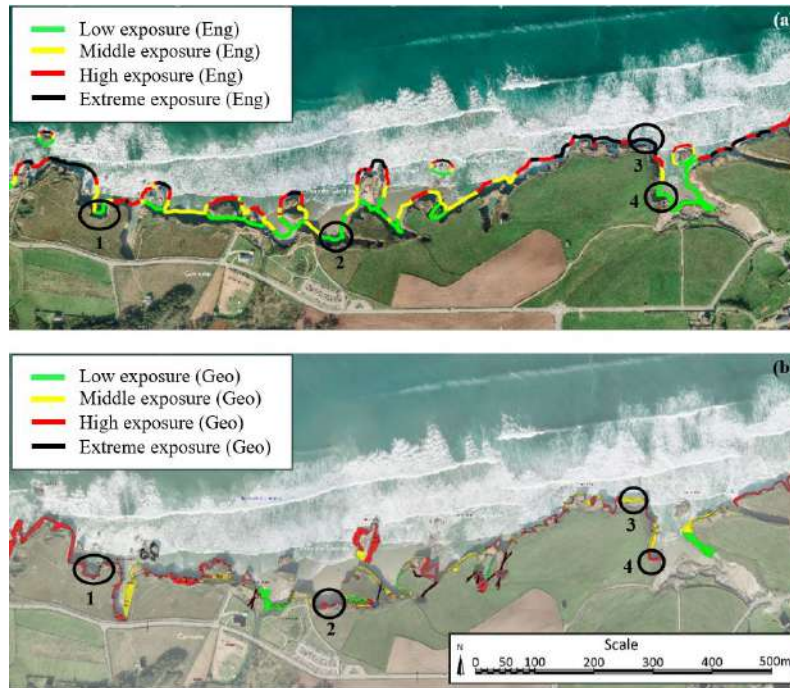


Figure 12: Comparison of exposure levels based on engineering (a) and geomorphological (b) approaches.

293 6. Conclusions

294 This work proposes a multidisciplinary approach for the characterization of
295 coastal cliffs based on engineering and geomorphological points of view. The
296 combined approach was applied to a study site in northwest Spain.

297 The engineering approach was characterized through wave power values
298 along the cliff toe, which were obtained by combining statistical approach and
299 wave propagation modelling. The results of the engineering approach reveal the
300 high variability in the wave power values along the cliff toe. That irregular dis-
301 tribution stems from: (1) the varying orientation of the shoreline with respect to
302 the prevailing wave direction, so that some cliffs are more exposed than others
303 to the incoming waves, and (2) the complex bathymetry at the nearshore re-

304 gion, which controls wave shoaling and refraction. On the basis of wave power,
305 four exposure levels, from low to extreme, were established and mapped. For
306 instance, the windward faces of the islets in front of the main shoreline present
307 high or extreme exposure levels, whereas the leeward faces of the islets and the
308 inlets along the main shoreline have low exposure levels.

309 The geomorphological approach was based on the Cliff Stability (CS) in-
310 dex, a new parameter developed *ad hoc* for this work, which may be applied to
311 characterise other cliffs elsewhere. The CS index is a function of the cliff ge-
312 ometry, lithology, structure and degradation state along with the hydrological
313 conditions. For the analysed case study, the CS index is more appropriate than
314 the existing Coastal Vulnerability Index (CVI), since the CVI does not include
315 some variables which play a relevant role in the particular case of the Catedrales
316 cliffs, such as the macro and microstructure of the rock formations, the rainfall
317 and runoff from the crowning of the cliff, the sediment type at the cliff toe, the
318 wave exposure level and the degradation state of the cliffs. The values of the CS
319 index served to define and map four exposure levels (from low to extreme, as
320 in the engineering approach) onto the study area. Most of the study area was
321 categorized as high exposure, except the inlets and the central stretch of beach.

322 The comparison of both approaches reveals similar results in some zones,
323 such as western boundaries of the study area or the north face of the Xangal
324 Islet. However, differences were observed in other areas, such as the west bank
325 of the inlet near the eastern boundary of the study area or the leeward face
326 of the Xangal Islet. The conclusion that may be drawn from these differences
327 is that wave power is important in shaping the cliff, but the rest of elements
328 contained in the CS index (geometry, lithology, structure, degradation state or
329 precipitation) also play a role. These results highlight the interest of combining
330 the two approaches, engineering and geomorphological, in characterising cliff
331 coasts, and the usefulness of the novel Cliff Stability (CS) index. This combined
332 approach, which is extensible to other coastal cliff environments elsewhere, may
333 be used as a management tool, in particular for the prevention of material and
334 human damages by determining the most exposed zones.

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